

# STRUCTURAL CALCULATIONS

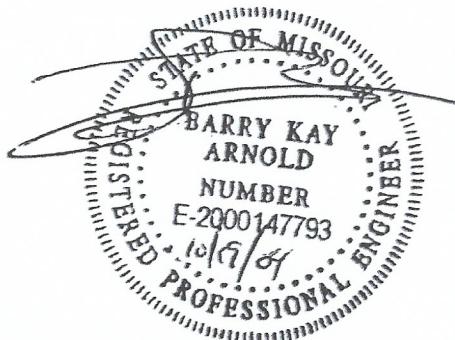
FOR

## Boman and Kemp Window Well

56" x 36" x 6'0"

Soil Pressure

45 psf/ft

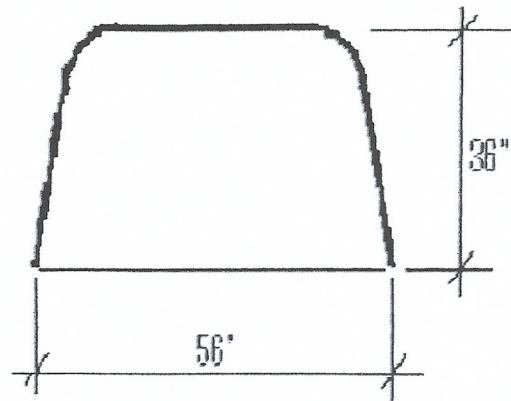


Prepared by

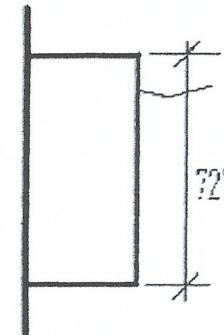
## ARW Engineers

1594 West Park Circle  
Ogden, UT 84404

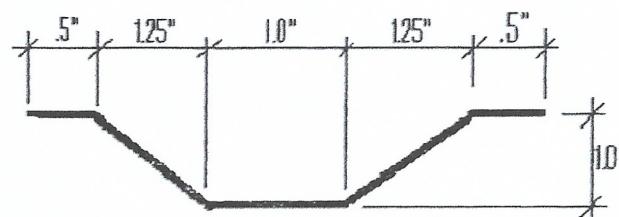
WINDOW WELL 56" x 36" x 6"-0"



Window Well Section



Window Well Section



Partial Section At Straight Elements

Section Properties	12 " width
thickness	$t_s := .043$
mom of inertia	$I_s := .088$
radius of gyr	$r := .384$
area	$a_s := .5965$
section mod	$s_s := .176$



Partial Section at Curved Elements

Section Properties	12 " width
thickness	$t_c := .043$
mom of inertia	$I_c := .0747$
radius of gyr	$r_c := .346$
area	$a_c := .625$
section mod	$s_c := .1436$

Material Properties

ASTM A572

$F_y := 42$  ksi

Stress Analysis for Soil Load of 45 psf/ft (Maximum Soil Load 270 psf)

Section forces from finite element analysis

$$\text{Maximum forces in straight section} \quad M_{z2} := 0.141 \text{ k-ft} \quad A_{x2} := 0.60 \text{ k}$$

$$f_{b2} := \frac{M_{z2} \cdot 12}{s_s} \quad f_{b2} = 9.614 \quad F_b := .667 \cdot F_y \quad F_b = 28.014$$

$$\text{let } kl := 1.0 \cdot 24 \quad \text{length between inflection points}$$

$$\frac{kl}{r} = 62.5 \quad c_c := \sqrt{2 \cdot \pi^2 \cdot \frac{29000}{42}} \quad c_c = 116.745 \quad \frac{kl}{c_c} = 0.535$$

$$F_a := .384 \cdot 42 \quad F_a = 16.128 \quad f_a := \frac{A_{x2}}{a_s} \quad f_a = 1.006$$

$$\text{Combined stresses} \quad csr := \frac{f_a}{F_a} + \frac{f_{b2}}{F_b} \quad csr = 0.406$$

$$\text{Maximum soil load capacity} \quad \text{Maxso2} := \frac{1.0}{csr} \cdot 270 \quad \text{Maxso2} = 665.778$$

Straight section

Check Curved Segment

Section forces from finite element analysis with snow load of 100psf

$$\text{Maximum forces in segment M23} \quad M_{zc2} := .077 \quad A_{xc2} := 0.69$$

$$f_{ac} := \frac{A_{xc2}}{a_c} \quad f_{ac} = 1.104$$

From AISI Handbook Allowable axial stress use  $F_y/2$  when  $d/r < 294$

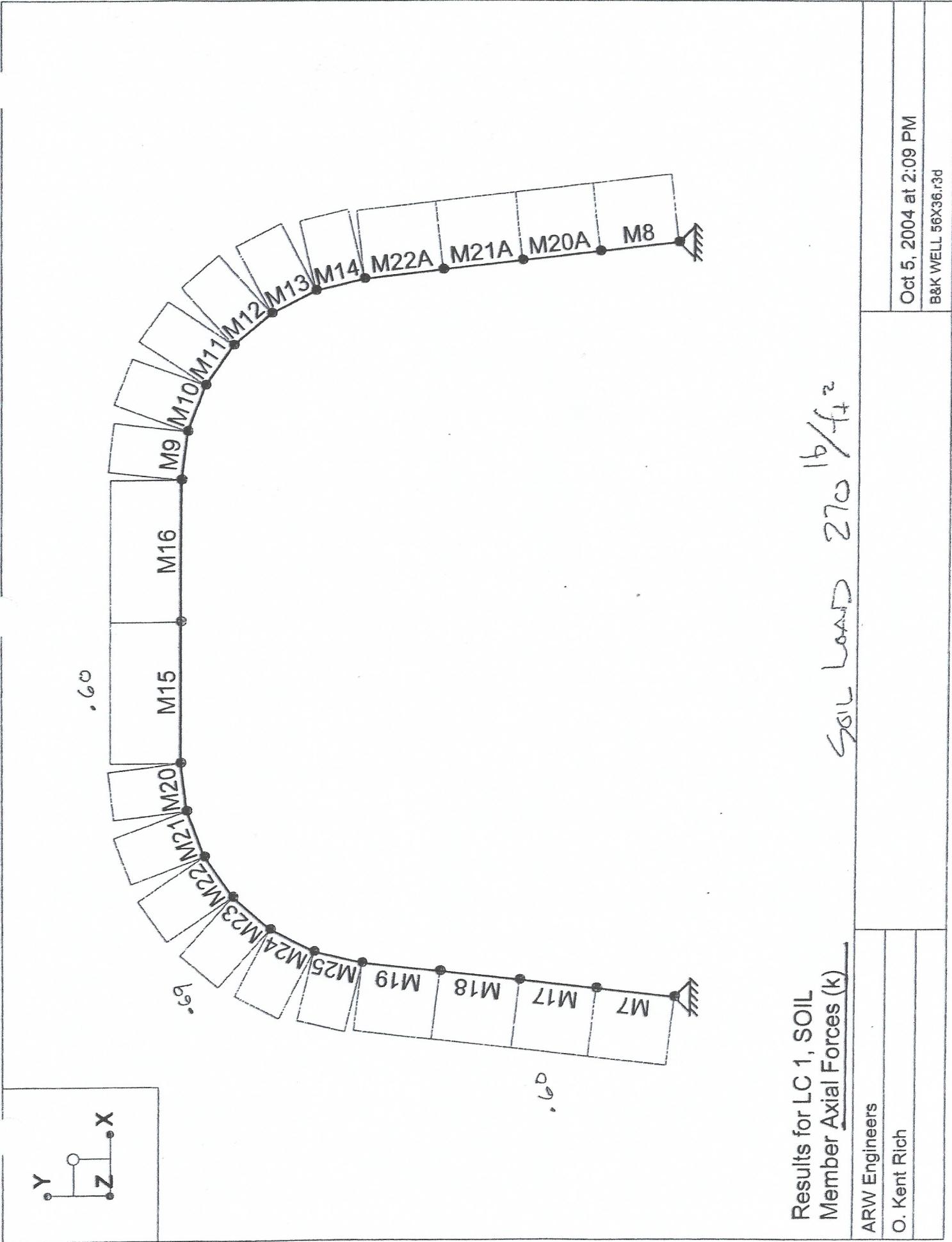
$$d := 26 \quad \frac{d}{r_c} = 75.145 \quad \text{Therefore use} \quad F_{ac} := \frac{F_y}{2}$$

$$f_{bc} := \frac{M_{zc2} \cdot 12}{s_c} \quad f_{bc} = 6.435$$

$$\text{Combined stresses} \quad csr_c := \frac{f_{ac}}{F_{ac}} + \frac{f_{bc}}{F_b} \quad csr_c = 0.282$$

$$\text{Maximum soil load capacity} \quad \text{Maxsol} := \frac{1.0}{csr_c} \cdot 270 \quad \text{Maxsol} = 956.559$$

Curved section



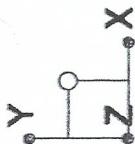
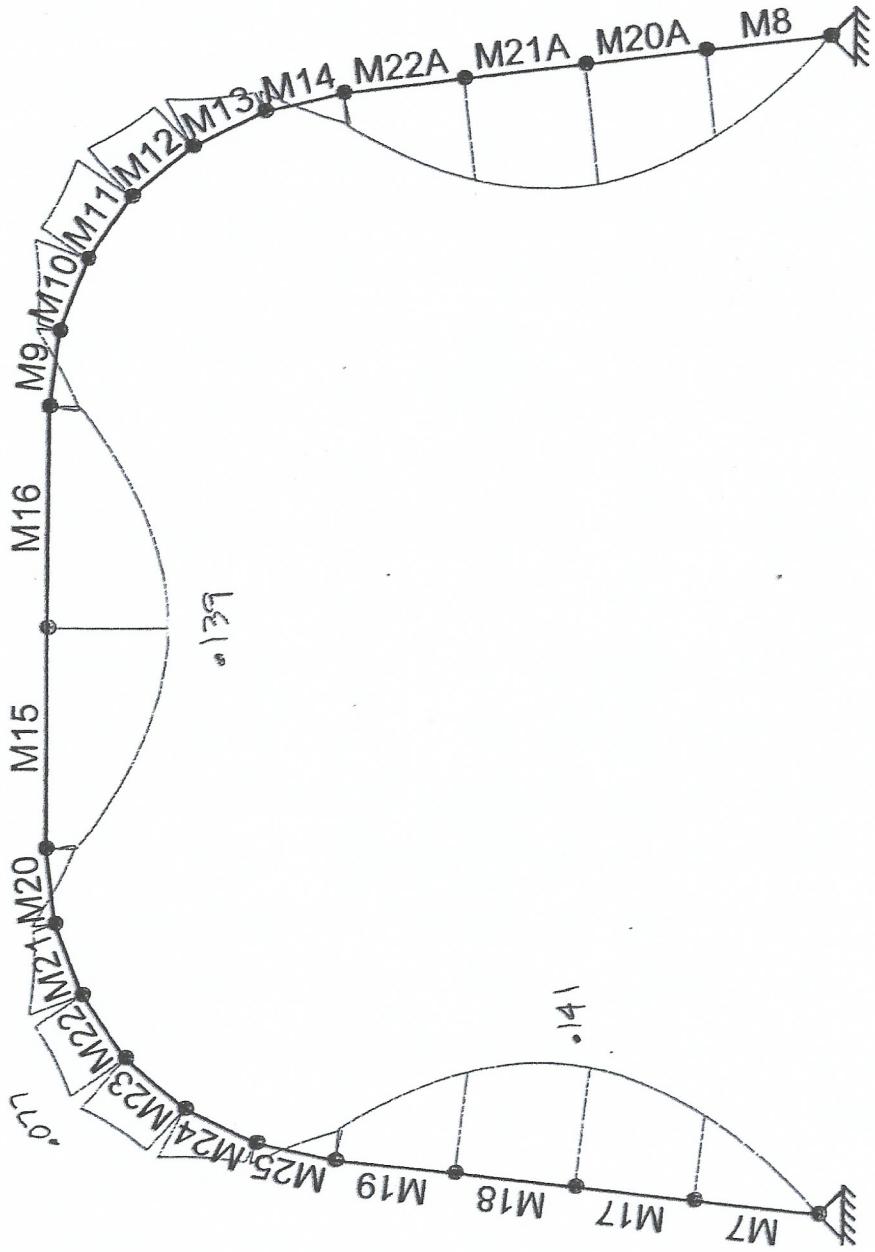
## Results for LC 1, SOIL Member Axial Forces (k)

ARW Engineers

O. Kent Rich

Oct 5, 2004 at 2:09 PM

B&K WELL 56X36,r3d



Results for LC 1, SOIL  
Member z Bending Moments (k-ft)

ARW Engineers

O. Kent Rich

Soil Load 270 lb/ft<sup>2</sup>

Oct 5, 2004 at 2:09 PM

B&K WELL 56X36.r3d